



**ÚSTÍ NAD ORLICÍ - OBNOVA VODOVODU  
V UL. VRBENSKÉHO  
K.Ú. ÚSTÍ NAD ORLICÍ**

**E.4 Dokladová část – statika potrubí**

## Zadávací podmínky

### Použité potrubí

PN 16, SN 16, De 90

Krytí nad vrcholem potrubí: 0,8 m

Zatížení provozem: D 400

Hladina spodní vody: 0,5 m pod povrchem

Obsypový materiál: lomová prosívka 0-8

Stupeň zhutnění obsypu: 95% PS

### Result

Calculation OK

### Given values

Pipe type		Pipe dimension (mm)	90
Soil type	Sand	Please note: the chosen pipe diameter is below 110 mm, but has been set at 110 mm for calculation purpose	
Safety class	Normal	Control class	Normal
Partial coefficient - safety class	2.27	Partial coefficient - control class	1.50
Max. negative pressure in pipe (kPa)	Compression class Normal > 95% SP		
Installation type	Normal trench and normal up to high compaction	Installation factor %	1.0 %
Bedding/bedding layer	Normal levelling layer	Bedding factor %	2.00 %
Traffic load	Heavy traffic load	Max. negative pressure in pipe (kPa)	0.00
Soil cover above pipe top (m) = H	0.80	Distance from ground level to ground-water level (m) = H <sub>w</sub>	0.50
Relative density - below ground-water level (kN/m <sup>3</sup> )	10.00	Relative density - above ground-water level (kN/m <sup>3</sup> )	20.00
Calculated diameter of pipe (mm)	90.00	Pipe ring stiffness	16.00

### Load combination 1.1 Deformation calculation serviceability limit state

Average stress from traffic load (q <sub>tm</sub> ) kN/m <sup>2</sup>	63.19	Short-term deformation from variable load (traffic)	1.4 %
Load factor C regarding the stiffness ratio of pipe to backfilling material (applied)	1.14	Short-term deformation from permanent load (soil)	0.4 %
Characteristic traffic load q <sub>tk</sub> kN/m <sup>2</sup>	72.25	Deformation from installation	1.0 %

(Formula 9)

Additional soil cover for determination of soil modulus  $E_{td}$  when influenced by heavy road traffic load (Table 2.8) 3.00

delta H factor dependent on type of road traffic load 1.0

Tangent modulus of backfill above ground-water  $E_{td}$  (Formula 11) -  $\text{kN/m}^2$  5376

Secant modulus of backfill above ground-water  $E_{sd}$  (Formula 12) -  $\text{kN/m}^2$  3495

Reduction factor for ground-water influence on soil E-moduli (formula 13) 0.85

Tangent modulus of backfill below ground-water  $E_{td}$  (Formula 11 x Formula 13) -  $\text{kN/m}^2$  4570

Secant modulus of backfill below ground-water  $E_{sd}$  (Formula 12 x Formula 13) -  $\text{kN/m}^2$  2971

(Table 2.9)

Average deformation 2.8 %

Short-term maximum deformation (Page 42) **4.8 %**

Long-term deformation from load (formula 16) 2.7 %

Long-term max. deformation (Formula 15) 6.6 %

**Short-term maximum deformation (Page 42) 4.8 % < 9.0 % ( Pipematerial: PE100) - OK**

**Load deformation 2.1 Deformation calculation ultimate limit state**

Calculated ring stiffness ( $\text{kN/m}^2$ )	14.00	Calculated max. buckling pressure ( $\text{kN/m}^2$ )	949.38
Calculated tangent modulus ( $\text{kN/m}^2$ )	2016.77	Design load ( $\text{kN/m}^2$ )	110.38
Reduction factor beta	0.80	Design buckling pressure ( $\text{kN/m}^2$ )	761.80

**Buckling load combination 2.1 -  $q_d$  (Formula 20)  $\text{kN/m}^2$  110.38 < Buckling load combination 2.1 -  $\beta x q_b$  (Formula 22) 761.80 - OK**

Při dodržení zadávacích podmínek potrubí PN 16, SN 16, De 90 vyhoví a jeho deformace nepřesáhne hodnotu 4,8 %.

Tato deformace je počítána při prázdném potrubí bez vnitřního přetlaku.

## Zadávací podmínky

**Použité potrubí:** Mondial PVC-O, PN 16, SN 16, De 90

**Krytí nad vrcholem potrubí:** 0,8 m

**Zatížení provozem:** D 400

**Hladina spodní vody:** 2 m pod povrchem

**Obsypový materiál:** lomová prosívka 0-8

**Stupeň zhuštění obsypu:** 95% PS

### Result

Calculation OK

### Given values

<b>Pipe type</b>	Mondial PVC-O	<b>Pipe dimension (mm)</b>	90
<b>Soil type</b>	Sand	<b>Please note: the chosen pipe diameter is below 110 mm, but has been set at 110 mm for calculation purpose</b>	
<b>Safety class</b>	Normal	<b>Control class</b>	Normal
<b>Partial coefficient - safety class</b>	2.27	<b>Partial coefficient - control class</b>	1.50
<b>Max. negative pressure in pipe (kPa)</b>	Compression class Normal > 95% SP		
<b>Installation type</b>	Normal trench and normal up to high compaction	<b>Installation factor %</b>	1.0 %
<b>Bedding/bedding layer</b>	Normal levelling layer	<b>Bedding factor %</b>	2.00 %
<b>Traffic load</b>	Heavy traffic load	<b>Max. negative pressure in pipe (kPa)</b>	0.00
<b>Soil cover above pipe top (m) = H</b>	0.80	<b>Distance from ground level to ground-water level (m) = Hw</b>	2.00
<b>Relative density - below ground-water level (kN/m³)</b>	10.00	<b>Relative density - above ground-water level (kN/m³)</b>	20.00
<b>Calculated diameter of pipe (mm)</b>	90.00	<b>Pipe ring stiffness</b>	16.00
<b><u>Load combination 1.1 Deformation calculation serviceability limit state</u></b>			
<b>Average stress from traffic load (q<sub>tm</sub>) kN/m²</b>	63.19	<b>Short-term deformation from variable load (traffic)</b>	0.9 %
<b>Load factor C regarding the stiffness ratio of pipe to backfilling material (applied)</b>	1.14	<b>Short-term deformation from permanent load (soil)</b>	0.2 %

Characteristic traffic load $q_{tk}$ kN/m <sup>2</sup> (Formula 9)	72.25	Deformation from installation (Table 2.9)	1.0 %
Additional soil cover for determination of soil modulus $E_{td}$ when influenced by heavy road traffic load (Table 2.8)	3.00	Average deformation	2.2 %
delta H factor dependent on type of road traffic load	1.0	Short-term maximum deformation (Page 42)	<u>4.2 %</u>
Tangent modulus of backfill above ground-water $E_{td}$ (Formula 11) - kN/m <sup>2</sup>	5376	Long-term deformation from load (formula 16)	1.8 %
Secant modulus of backfill above ground-water $E_{sd}$ (Formula 12) - kN/m <sup>2</sup>	3495	Long-term max. deformation (Formula 15)	5.3 %
Reduction factor for ground-water influence on soil E-moduli (formula 13)	1.60		
Tangent modulus of backfill below ground-water $E_{td}$ (Formula 11 x Formula 13) - kN/m <sup>2</sup>	8602		
Secant modulus of backfill below ground-water $E_{sd}$ (Formula 12 x Formula 13) - kN/m <sup>2</sup>	5592		

**Short-term maximum deformation (Page 42) 4.2 % < 9.0 % ( Pipematerial: PE100) - OK**

**Load deformation 2.1 Deformation calculation ultimate limit state**

Calculated ring stiffness (kN/m <sup>2</sup> )	14.00	Calculated max. buckling pressure (kN/m <sup>2</sup> )	1029.75
Calculated tangent modulus (kN/m <sup>2</sup> )	3796.28	Design load (kN/m <sup>2</sup> )	133.93
Reduction factor beta	0.84	Design buckling pressure (kN/m <sup>2</sup> )	864.72

**Buckling load combination 2.1 -  $q_d$  (Formula 20) kN/m<sup>2</sup> 133.93 < Buckling load combination 2.1 -  $\beta x q_b$  (Formula 22) 864.72 - OK**

Při dodržení zadávacích podmínek potrubí Mondial PVC-O, PN 16, SN 16, De 90 vyhoví a jeho deformace nepřesáhne hodnotu 4,2 %.  
Tato deformace je počítána při prázdném potrubí bez vnitřního přetlaku.

## Zadávací podmínky

**Použité potrubí:**

**PN 16, SN 16, De 90**

**Krytí nad vrcholem potrubí:** 1,5 m

**Zatížení provozem:** D 400

**Hladina spodní vody:** 0,5 m pod povrchem

**Obsypový materiál:** lomová prosívka 0-8

**Stupeň zhutnění obsypu:** 95% PS

### Result

Calculation OK

### Given values

<b>Pipe type</b>		<b>Pipe dimension (mm)</b>	90
<b>Soil type</b>	Sand	<b>Please note: the chosen pipe diameter is below 110 mm, but has been set at 110 mm for calculation purpose</b>	
<b>Safety class</b>	Normal	<b>Control class</b>	Normal
<b>Partial coefficient - safety class</b>	2.27	<b>Partial coefficient - control class</b>	1.50
<b>Max. negative pressure in pipe (kPa)</b>	Compression class Normal > 95% SP		
<b>Installation type</b>	Normal trench and normal up to high compaction	<b>Installation factor %</b>	1.0 %
<b>Bedding/bedding layer</b>	Normal levelling layer	<b>Bedding factor %</b>	2.00 %
<b>Traffic load</b>	Heavy traffic load	<b>Max. negative pressure in pipe (kPa)</b>	0.00
<b>Soil cover above pipe top (m) = H</b>	1.50	<b>Distance from ground level to ground-water level (m) = H<sub>w</sub></b>	0.50
<b>Relative density - below ground-water level (kN/m<sup>3</sup>)</b>	10.00	<b>Relative density - above ground-water level (kN/m<sup>3</sup>)</b>	20.00
<b>Calculated diameter of pipe (mm)</b>	90.00	<b>Pipe ring stiffness</b>	16.00

### Load combination 1.1 Deformation calculation serviceability limit state

<b>Average stress from traffic load (q<sub>tm</sub>) kN/m<sup>2</sup></b>	30.29	<b>Short-term deformation from variable load (traffic)</b>	0.6 %
<b>Load factor C regarding the stiffness ratio of pipe to backfilling material (applied)</b>	1.03	<b>Short-term deformation from permanent load (soil)</b>	0.6 %
<b>Characteristic traffic load q<sub>tk</sub> kN/m<sup>2</sup> (Formula 9)</b>	31.24	<b>Deformation from installation (Table 2.9)</b>	1.0 %
<b>Additional soil cover for</b>	1.55	<b>Average deformation</b>	2.2 %

determination of soil modulus  $E_{td}$   
when influenced by heavy road  
traffic load (Table 2.8)

delta H factor dependent on type of road traffic load	1.0	Short-term maximum deformation (Page 42)	<u>4.2 %</u>
Tangent modulus of backfill above ground-water $E_{td}$ (Formula 11) - $\text{kN/m}^2$	4552	Long-term deformation from load (formula 16)	1.8 %
Secant modulus of backfill above ground-water $E_{sd}$ (Formula 12) - $\text{kN/m}^2$	2959	Long-term max. deformation (Formula 15)	5.4 %
Reduction factor for ground-water influence on soil E-moduli (formula 13)	0.73		
Tangent modulus of backfill below ground-water $E_{td}$ (Formula 11 x Formula 13) - $\text{kN/m}^2$	3338		
Secant modulus of backfill below ground-water $E_{sd}$ (Formula 12 x Formula 13) - $\text{kN/m}^2$	2170		

**Short-term maximum deformation (Page 42) 4.2 % < 9.0 % ( Pipematerial: PE100) - OK**

**Load deformation 2.1 Deformation calculation ultimate limit state**

Calculated ring stiffness ( $\text{kN/m}^2$ )	14.00	Calculated max. buckling pressure ( $\text{kN/m}^2$ )	811.36
Calculated tangent modulus ( $\text{kN/m}^2$ )	1473.00	Design load ( $\text{kN/m}^2$ )	71.06
Reduction factor beta	0.84	Design buckling pressure ( $\text{kN/m}^2$ )	678.88

**Buckling load combination 2.1 -  $q_d$  (Formula 20)  $\text{kN/m}^2$  71.06 < Buckling load combination 2.1 -  $\beta x q_b$  (Formula 22) 678.88 - OK**

Při dodržení zadávacích podmínek potrubí PN 16, SN 16, De 90 vyhoví a jeho deformace nepřesáhne hodnotu 4,2 %.  
Tato deformace je počítána při prázdném potrubí bez vnitřního přetlaku.

## Zadávací podmínky

**Použité potrubí: Mondial PVC-O, PN 16, SN 16, De 90**

**Krytí nad vrcholem potrubí: 1,5 m**

**Zatížení provozem: D 400**

**Hladina spodní vody: 2 m pod povrchem**

**Obsypový materiál: lomová prosívka 0-8**

**Stupeň zhutnění obsypu: 95% PS**

### Result

Calculation OK

### Given values

Pipe type	Mondial PVC-O	Pipe dimension (mm)	90
Soil type	Sand	Please note: the chosen pipe diameter is below 110 mm, but has been set at 110 mm for calculation purpose	
Safety class	Normal	Control class	Normal
Partial coefficient - safety class	2.27	Partial coefficient - control class	1.50
Max. negative pressure in pipe (kPa)	Compression class Normal > 95% SP		
Installation type	Normal trench and normal up to high compaction	Installation factor %	1.0 %
Bedding/bedding layer	Normal levelling layer	Bedding factor %	2.00 %
Traffic load	Heavy traffic load	Max. negative pressure in pipe (kPa)	0.00
Soil cover above pipe top (m) = H	1.50	Distance from ground level to ground-water level (m) = H <sub>w</sub>	2.00
Relative density - below ground-water level (kN/m <sup>3</sup> )	10.00	Relative density - above ground-water level (kN/m <sup>3</sup> )	20.00
Calculated diameter of pipe (mm)	90.00	Pipe ring stiffness	16.00

### Load combination 1.1 Deformation calculation serviceability limit state

Average stress from traffic load (q <sub>tm</sub> ) kN/m <sup>2</sup>	30.29	Short-term deformation from variable load (traffic)	0.5 %
Load factor C regarding the stiffness ratio of pipe to backfilling material (applied)	1.03	Short-term deformation from permanent load (soil)	0.5 %
Characteristic traffic load q <sub>tk</sub> kN/m <sup>2</sup> (Formula 9)	31.24	Deformation from installation (Table 2.9)	1.0 %
Additional soil cover for determination of soil modulus E <sub>td</sub>	1.55	Average deformation	2.0 %



when influenced by heavy road traffic load (Table 2.8)

delta H factor dependent on type of road traffic load	1.0	Short-term maximum deformation (Page 42)	<u>4.0 %</u>
Tangent modulus of backfill above ground-water $E_{td}$ (Formula 11) - $\text{kN/m}^2$	4552	Long-term deformation from load (formula 16)	1.4 %
Secant modulus of backfill above ground-water $E_{sd}$ (Formula 12) - $\text{kN/m}^2$	2959	Long-term max. deformation (Formula 15)	4.9 %
Reduction factor for ground-water influence on soil E-moduli (formula 13)	1.13		
Tangent modulus of backfill below ground-water $E_{td}$ (Formula 11 x Formula 13) - $\text{kN/m}^2$	5158		
Secant modulus of backfill below ground-water $E_{sd}$ (Formula 12 x Formula 13) - $\text{kN/m}^2$	3353		

**Short-term maximum deformation (Page 42) 4.0 % < 9.0 % ( Pipematerial: PE100) - OK**

**Load deformation 2.1 Deformation calculation ultimate limit state**

Calculated ring stiffness ( $\text{kN/m}^2$ )	14.00	Calculated max. buckling pressure ( $\text{kN/m}^2$ )	947.47
Calculated tangent modulus ( $\text{kN/m}^2$ )	2276.46	Design load ( $\text{kN/m}^2$ )	80.61
Reduction factor beta	0.85	Design buckling pressure ( $\text{kN/m}^2$ )	807.52

**Buckling load combination 2.1 -  $q_d$  (Formula 20)  $\text{kN/m}^2$  80.61 < Buckling load combination 2.1 -  $\beta x q_b$  (Formula 22) 807.52 - OK**

Při dodržení zadávacích podmínek potrubí Mondial PVC-O, PN 16, SN 16, De 90 vyhoví a jeho deformace nepřesáhne hodnotu 4,0 %.  
Tato deformace je počítána při prázdném potrubí bez vnitřního přetlaku.